Enhancement of Durability Properties and Drying Shrinkage of Heat-treated Oil Palm Shell Species High-strength Lightweight Concrete
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Abstract: Aim: In this study, the effects of heat-treated and non-treated oil palm shell (OPS) species (dura and tenera) are investigated on the slump, density and compressive strength of oil palm shell concrete (OPSC). Two different species of OPS coarse aggregates are subjected to heat treatment at 65 and 130 °C with the duration of 1 h. The results show that the workability of the OPSC increases significantly with an increase in temperature of heat-treated of the tenera OPS aggregates. It is found that the maximum achievable 28-days and 180-days compressive strength is 51 and 54 MPa, respectively. Furthermore, rapid chloride penetration tests (RCPT), porosity measurement and water absorption tests were performance to signify the effects of heat treatment on different OPS species lightweight concrete (LWC). The use of heat-treated OPS LWC induced the advantages of reducing the permeability and capillary porosity as well as water absorption. The results showed that the ideal of heat treatment method has enhanced the performance of drying shrinkage. Hence, the findings of this study are of primary importance as they revealed that the heat treatment on OPS species LWC can be used as a new environmentally friendly method to enhance the durability properties and drying shrinkage of OPS LWC.

Keywords: Lightweight concrete; oil palm shells; rapid chloride penetration test; water absorption; drying shrinkage

1. Introduction

The trend towards modernization in construction industry has caused to a rapid growth and huge demand for concrete. The vast demand for concrete has resulted in the over-exploitation of natural stone deposit such as granite and gravel. As a result of escalating environments problem, it has severely affected the stability of the construction industry. Thus, causing ecological imbalance and the need to find an alternative solution to replace this conventional aggregate material has become necessary. One such alternative is oil palm shell (OPS), which is a form of agricultural solid waste of oil palm manufacturing process. OPS will become an environmental problem if no effort is made to utilise it. Therefore, the oil palm industry and concrete industry must be prepared to take advantage of this situation and utilize the available oil palm waste materials in the best possible way, to solve disposal problems and create value-added products.

The oil palm is a major tree crop growing in regions where the temperature is hot with coastal rain forest such as Malaysia, Indonesia, and Thailand in Southeast Asia, Nigeria in Africa, Colombia and Ecuador in South America, and Papua New Guinea in Oceania (UNEP, 2011). The species of palm tree Elaeis guineensis Jacq was taken to Malaysia from Eastern Nigeria in 1961. The best known of several types of Elaeis guineensis are: Dura, Tenera and Pisifera. Dura is a homozygous dominant with thick shells while pisifera is a homozygous recessive without shells. A ring of fibres called “mesocarp” surrounds the kernel. Dura and Pisifera are crossed to produce Tenera (Janssens, 1927) hybrid seed. Recently, Yew et al. (2014) found that compressive strength for LWC using dura OPS increases significantly compared to tenera OPS, which was 21.8%. In this study, dura OPS with heat treatment is selected and used as the coarse
aggregate for producing high strength lightweight concrete.

The utilization of OPS as LWA in the production of lightweight aggregate concrete (LWAC) has been a topic of research since early 1984 in Malaysia by Abdullah (1984). Structural lightweight concrete (SLWC) has been used for many years, it can be said that SLWC or high strength lightweight aggregate concrete (HSLWAC) is similar to normal lightweight concrete (NLC) except their density is lower. The density of SLWC typically ranges between 1400 and 2000 kg/m$^3$ compared with that of 2400 kg/m$^3$ for NWC (Shannag, 2011). Typically, high strength lightweight aggregate concrete (HSLWAC) has a compressive strength levels between 34 and 69 MPa. HSLWAC has an air-dry density less than 2000 kg/m$^3$ and water to cement ratio of less than 0.45 (Hoff, 2002). The density of the dura and tenera shells is within the range of a majority lightweight aggregates and the specific gravity ranges between 1.15 and 1.37 g/cm$^3$ (Yew et al., 2014). Results have shown that, the 28-day air-dry density and oven-dry density for concrete produced from OPS aggregates are approximately 16 and 20% lower than that of ordinary concrete, respectively (Yew et al., 2014).

Concretes, typically lightweight concretes have been used in construction and its benefit include reduction in dead loads; savings in foundations and reinforcement; improved thermal properties, improved fire resistance; savings in transporting and handling precast units on site; and reduction in formwork and propping. At present, lightweight aggregate concrete can be produced using a variety of lightweight aggregates. Lightweight aggregates can originate from either natural materials, like volcanic pumice; thermal treatment of natural raw materials like clay, slate or shale; manufactured from industrial by-products, such as fly ash; or processing of industrial by-products, such as slag. These types of commercially available lightweight aggregates are obtained through a very high heat treatment method at 1000-1200 ° C and results in high fuel costs (Zhang & Gjorv, 1991). Therefore, towards achieving sustainable environmental and saving in the overall cost of construction, many plant based waste material resources have been researched as potential to be aggregates in the production of lightweight concrete. In Malaysia, it is estimated that over 4.6 million tonnes of OPS as waste materials is produced annually (Teo et al., 2006). Research has already been conducted in order to utilize and improve the economic value of OPS residue by producing ‘OPS hollow blocks’ for walls and ‘OPS concrete’ for footings, lintels and beams to achieve affordable and comfortable houses (Teo et al., 2007).

OPS is an organic in nature, its properties may degrade after a certain period of time by fungal decay and termite attack unless pre-treatment is applied on the aggregates. Yew et al. (2014) reported that heat-treated method can be used as a new eco-friendly alternative compared to chemically impregnated on OPS aggregates (Mannan et al., 2006). Therefore, heat-treated can be used as a means to increase the resistance against fungal decay and insect attack and subsequently improve the properties of raw OPS aggregates and the resulting concrete properties. Recently, OPS HSLWAC with a corresponding 28-day compressive strength of more than 40 MPa and a density of about 2000 kg/m$^3$ have been successfully produced compared to normal strength LWAC (Shafigh et al., 2011, Islam et al., 2016, Yew et al., 2015). In current research, there have no experimental researches have been performed on improving durability properties and drying shrinkage of OPSC, particularly by incorporating heat-treated and non-treated OPS species (dura and tenera) to achieve high strength lightweight concrete. Therefore, the objective of this paper discussed the durability properties and drying shrinkage performance of heat-treated and non-treated OPSC were investigated by means of drying shrinkage, rapid chloride penetration test (RCPT), porosity measurement, water absorption and drying shrinkage tests.

2. Materials and Methods

2.1. Materials

2.1.1 Cement

The cement used in the mixing was Ordinary Portland Cement (OPC) Type 1 (ASTM, 1990). The cement was manufactured by Tasek Corporation Berhard with a specific gravity of 3.14 g/cm$^3$. The Blaine’s specific surface area for this cement was 3510 cm$^2$/g. The cement content was kept constant at 545 kg/m$^3$.

2.1.2 Water and superplasticizer (SP)
Potable water with pH value of 6 was used for both mixing and curing. The water to cement ratio of 0.29 was used for all the mixes. The SP used in this study was polycarboxylic ether (PCE) supplied by BASF, which complies with ASTM: C494/C494M-13. The SP was added in all the mixes at a constant amount of 1.0% of the cement weight in order to facilitate workability.

2.1.3 Fine and coarse aggregate

Local mining sand available from Hanson Quarry Selangor was used as the fine aggregate, having a specific gravity, fineness modulus, water absorption and maximum grain size of 2.68, 2.72, 0.97% and 4.75 mm, respectively. The fine aggregate was dried in an open air before use and was kept constant at 866 kg/m³ for all mixes.

In this study, crushed dura and tenera OPS species was used as the coarse aggregate as shown in Fig. 1. The dura and tenera OPS were collected from a local crude palm oil factory. The thickness of the OPS used throughout the research was 2.0 to 5.0 mm. Furthermore, OPS were washed and sieved using a 12.5 mm-sieve. The OPS aggregates that were retained in the sieve were crushed using a stone-crushing machine in the laboratory. The crushed dura OPS aggregates were sieved using a 9.5 mm-sieve to remove dura and tenera OPS aggregates with sizes more than 9.5 mm. For investigation of the size effect of OPS aggregate on compressive strength, crushed OPS aggregate sizes that were below 2.36 mm were removed and the sizes between 2.36 and 9.5 mm were used. The benefits of using crushed OPS aggregates in OPSC were reported by Yew et al. (2014). In this study, OPS aggregates were heat-treated at 65 °C and 130 °C over a period of 1 h using a temperature-controlled laboratory oven. The heat-treated OPS will be rapidly cooling down by soaking under the water at 22 ± 2 °C as shown in Fig. 2. The OPS aggregates were subsequently air dried in the laboratory to attain an approximately saturated surface dry condition. A comparison between the physical properties of heat-treated and without heat-treated of crushed OPS aggregates is presented in Table 1, while the grading of dura and tenera OPS aggregates is presented in Table 2. The OPS content in all the mixes was kept constant at 365 kg/m³.

Figure 1. Crushed (a) dura and (b) tenera OPS species between 2.36 and 9.5 mm

<table>
<thead>
<tr>
<th>Physical property</th>
<th>dura OPS</th>
<th>dura OPS*</th>
<th>tenera OPS</th>
<th>tenera OPS*</th>
</tr>
</thead>
<tbody>
<tr>
<td>Maximum size (mm)</td>
<td>9.5</td>
<td>9.5</td>
<td>9.5</td>
<td>9.5</td>
</tr>
<tr>
<td>Specific gravity (saturated surface dry)</td>
<td>1.32</td>
<td>1.29</td>
<td>1.28</td>
<td>1.25</td>
</tr>
<tr>
<td>Finess modulus</td>
<td>5.70</td>
<td>5.76</td>
<td>5.78</td>
<td>5.90</td>
</tr>
<tr>
<td>Compacted bulk density (kg/m³)</td>
<td>623</td>
<td>617</td>
<td>628</td>
<td>620</td>
</tr>
<tr>
<td>Water absorption (1 and 24 h) (%)</td>
<td>12.10 and 19.61</td>
<td>10.02 and 17.15</td>
<td>12.06 and 19.55</td>
<td>9.96 and 17.08</td>
</tr>
<tr>
<td>Aggregate crushing value (%)</td>
<td>2.35</td>
<td>2.22</td>
<td>2.20</td>
<td>2.11</td>
</tr>
</tbody>
</table>

Table 1. Comparison of the physical properties between heat-treated and without heat-treated dura and tenera OPS aggregates
2.2. Mix proportions

A total of 6 mixes were prepared. The mix proportions of all the concrete mixes used in this study are presented in Table 3. The dosage of water and superplastisizer were kept constant for all mixes.

<table>
<thead>
<tr>
<th>No.</th>
<th>Mix code</th>
<th>Cement</th>
<th>Water</th>
<th>W/C ratio</th>
<th>Sand</th>
<th>OPS</th>
<th>Duration (h)</th>
<th>Temperature (°C)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>T0</td>
<td>505</td>
<td>146</td>
<td>0.29</td>
<td>865</td>
<td>-</td>
<td>368</td>
<td>0</td>
</tr>
<tr>
<td>2</td>
<td>D0</td>
<td>505</td>
<td>146</td>
<td>0.29</td>
<td>865</td>
<td>-</td>
<td>368</td>
<td>0</td>
</tr>
<tr>
<td>3</td>
<td>T65/1</td>
<td>505</td>
<td>146</td>
<td>0.29</td>
<td>865</td>
<td>368</td>
<td>-</td>
<td>1</td>
</tr>
<tr>
<td>4</td>
<td>D65/1</td>
<td>505</td>
<td>146</td>
<td>0.29</td>
<td>865</td>
<td>368</td>
<td>-</td>
<td>1</td>
</tr>
<tr>
<td>5</td>
<td>T130/1</td>
<td>505</td>
<td>146</td>
<td>0.29</td>
<td>865</td>
<td>368</td>
<td>-</td>
<td>1</td>
</tr>
<tr>
<td>6</td>
<td>D130/1</td>
<td>505</td>
<td>146</td>
<td>0.29</td>
<td>865</td>
<td>368</td>
<td>-</td>
<td>1</td>
</tr>
</tbody>
</table>

Table 3. Mixture proportion (kg/m³)

Note: HT = heat-treated, NT= non-treated

2.3. Testing methods

The procedure adopted for mixing the OPSC with heat-treated and non-treated OPS species involves the following steps. Firstly, the sand and OPS were poured into a concrete mixer and dry mixed for 1 min. Secondly, the cement was spread and dry mixed for 1 min. This is followed by the addition of water and superplasticizer with a mixing time of 5 min. Slump test was performed on the mixture prior to sample casting. The concrete specimens were cast in 100-mm cube steel oiled moulds, and a poker vibrator was used to eliminate air bubbles in the mixture. The specimens were demoulded approximately 24 hours after casting and were cured in water at 25 ± 2 °C until age of testing. The compression testing machine used was an ELE (Engineering Laboratory Equipment) with a load capacity of 3000 kN running at a pace rate of 3.0 kN/sec, in accordance to BS EN 12390-4:2000. To determine the mechanical properties for
each mixture, 18 cubes (100 × 100 × 100 mm) are used to determine the compressive strength at 1, 7, 28 and, 180 days.

Furthermore, to determine the durability of each mixture, four cylinders (diameter: 100 mm, height: 200 mm) were cut into three disc (diameter: 100 mm, height: 50 mm). According to the ASTM C-1202 standards, the resistance of the specimens to chloride ion penetration at the age of 7, 28 and 180 days were measured. Total charges passed after 6 h was obtained from integration of current over the test duration. In this study, porosity measurement was also conducted to measure all the specimens at the age of 28, 90 and 180 days in accordance with RILEM Recommendations (RILEM, 1984). For each mixture, three concrete cubes were cast in order to determine the average value at a particular age. To determine the water absorption of all mixes at the age of 28 days, specimens were dried in the oven at 100 ± 5 °C to reach a constant mass and then fully immersed in water with 22 ± 2 °C for 72 h. This test was similar to the test conducted by Shafigh et al. (2011) and Teo et al. (2007).

Drying shrinkage measurements were performed according to ASTM: C157-75M-04. The length change of prism 50 mm x 50 mm x 300 mm was measured by a dial gage extensometer with a 285 mm. Measurements were carried out every two days for the first two weeks, then every week for 1 month and every month up to 365 days.

3 Results and Discussion

3.1 Properties of fresh concrete (Workability)

In this study, the slump test was carried out to determine the consistency of fresh concrete. The slump value of fresh OPSC and heat-treated OPSC are presented in Fig. 4. The workability of concrete was fixed within a range of 100 – 210 mm for the slump test. In this study, the quantity of water and SP were kept constant for all mixes in order to access the effects of heat treatment on the workability of OPSC. The aims of heat-treated on different species of OPS is to improve the surface quality. From Fig. 3, it could be seen that the workability of fresh OPSC increases due to an increase of treatment temperature. The T65/1 and D65/1 increase the workability at about 12.9% and 20.0% compared to T0 and D0 without heat-treated on OPS. The lowest slump value of D0 may be due to lower surface quality and thicker dura OPS aggregate tend to absorb water compared to other mixes which affects workability. However, it was found that T130/1 and D130/1 mix exhibit a very high workability within a range of 202 – 210 mm. It can be observed that no segregation in these mixes, which can be attributed to the fact that highest temperature reduces water absorption on thinner tenera OPS compared to dura OPS which also leads to an increase in workability of concrete. Yew et al. (2014) reported that the workability of the concrete can be enhanced by increasing the heat-treated temperature and duration of OPS aggregates.

3.2 Density

Two types of density, namely, demoulded density (DD) and oven-dry density (ODD) were measured for all mixes. Structural lightweight concrete (SLWC) is typically defined as concrete with ODD of not greater than 2000 kg/m³ (Newman & Owens, 2003). It can be seen that all OPSC density having an ODD and DD within the range of 1921 – 1950 kg/m³ and 1993 – 2016 kg/m³ which fulfilled the requirement of structural LWC for all mixes. The DD and
ODD with the increasing of heat-treated temperature on dura OPS from NT to 65 °C/1 h and 130 °C/1 h (D0, D65/1 and D130/1) shown the marginal density reduction compared to tenera OPSC (T0, T65/1 and T130/1) at about 0.3%. This observation is attributed to the weight loss in OPS due to the heat treatment temperature. Other studies observed similar trend by drying wood at high temperature, this phenomenon is due to decrease in hygroscopicity of wood material (Stamm & Hansen, 1937 and Fengel, 1996).

![Figure 4: Temperature and duration of heat treatment on OPS species versus density](image)

### 3.3 Compressive strength

#### 3.3.1 Continuous moist curing

The results of the compressive strength for all concrete mixes subject to moist curing up to 180 days are presented in Table 4. It could be noted that all the compressive strength of heat-treated and non-treated OPSC increased at all ages, however marginal compressive strength gain at 180-days. Yew et al. (2014) reported that not all type of OPS species aggregates is suitable for production of HSLWC. Therefore, OPS with different species (dura and tenera) have been selected in this study. It also found that, the use of proper heat treatment method on crushed OPS aggregate caused notable surface quality improvement without compromising the strength of the OPSC.

Table 4 shows the compressive strength development for OPSC containing different species of OPS aggregates with heat-treated and non-treated. As can be seen, there is notable difference between D0 and T0 with the increment of 6.8% compressive strength of D0 compared to T0 at 28 days. The highest compressive strength was achieved for the D65/1 concrete mix, with a value of 50.6 and 53.5 MPa at 28-days and 180-days, respectively. However, the compressive strength decreased significantly with an increase in heat treatment temperature on tenera OPS from T0 to T130/1 at all ages. This may due to the rate of temperature and duration as well as thinner tenera OPS which influence the properties of OPS. On the other hand, a slight increase in compressive strength was observed from D0 to D130/1. Stated differently, the thicker dura OPS aggregate gain in compressive strength by increasing the temperature and duration of heat treatment, relatively to the tenera coarse aggregate with heat-treated and non-treated.

<table>
<thead>
<tr>
<th>Mix code</th>
<th>Compressive strength (MPa)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>1d</td>
</tr>
<tr>
<td>T0</td>
<td>40.86</td>
</tr>
<tr>
<td></td>
<td>(0.5)</td>
</tr>
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</table>
Table 4. Development of compressive strength of dura and tenera aggregates OPSC under continuous moist curing

<table>
<thead>
<tr>
<th></th>
<th>T0</th>
<th>D0</th>
<th>T65/1</th>
<th>D65/1</th>
<th>T130/1</th>
<th>D130/1</th>
</tr>
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<tbody>
<tr>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>41.25</td>
<td>45.5</td>
<td>46.66</td>
<td>47.05</td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>(0.3)</td>
<td>(0.1)</td>
<td>(0.3)</td>
<td>(0.2)</td>
<td></td>
<td></td>
</tr>
<tr>
<td>T65/1</td>
<td>41.12</td>
<td>44.25</td>
<td>45.18</td>
<td>46.62</td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>(0.4)</td>
<td>(0.3)</td>
<td>(0.5)</td>
<td>(0.2)</td>
<td></td>
<td></td>
</tr>
<tr>
<td>D65/1</td>
<td>45.62</td>
<td>50.31</td>
<td>50.60</td>
<td>53.50</td>
<td></td>
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<tr>
<td></td>
<td>(0.3)</td>
<td>(0.1)</td>
<td>(0.3)</td>
<td>(0.5)</td>
<td></td>
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<tr>
<td>T130/1</td>
<td>41.28</td>
<td>43.85</td>
<td>44.02</td>
<td>44.22</td>
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<td>(0.5)</td>
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<td>D130/1</td>
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<td>(0.6)</td>
<td>(0.5)</td>
<td>(0.6)</td>
<td>(0.5)</td>
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</tbody>
</table>

3.4 Durability Performance

3.4.1 Rapid chloride penetration test (RCPT)

![Figure 5](image)

The results of RCPT on 7-day, 28-day and 180-day water cured specimens are illustrated in Fig. 5. It can be seen that, the total transferred charge between the NaCl and NaOH cells with electrical potential of 60 V in 6 h at all heat-treated aggregate OPSC is significantly lower than that of non-treated aggregate OPSC (T0 and D0). The results show that heat-treated on dura aggregate improved the resistance of concrete to ion penetration. It might be attributed to reduction in inner conductivity of pores and less capillary porosity, which creates the bars of concrete to be more secured from corrosion. In accordance with ASTM C-1202 classification and the RCPT value, all of the heat-treated OPSFRC specimens are classified as “low” and “very low” permeability at 28-days and 180-days, respectively, except for T130/1 achieved above 2000 C considered as “moderator” permeability. From the results, it can be noted that the increase and decrease of charge passed along with the suitable temperature and duration of heat treatment (D65/1) as well as dura aggregates can be related to reduce in porosity which can cause the lower resistance to ion penetration rather than the specimen with higher temperature and duration of heat treatment (T130/1) of tenera species. Thus, it can be deduced that the (C) value measured for the heat-treated on dura aggregates OPSC in this study falls within the range of “low” and “very low” of chloride ion penetrability at 28-days and 180-days.
3.4.2. Porosity measurement

![Image of porosity measurement graph]

Figure 6: Porosity versus heat-treated and non-treated on different species aggregate OPSC

Porosity is one of the major parameters which influence the strength and durability of concrete. In order to measure the porosity of specimens, porosity measurement test was also performed in addition to RCPT test in this study. It can be related to low porosity cause the low ion chloride penetration rate which resulting of high durability delay the corrosion of specimens. The porosity values measured using the pressure saturation apparatus for OPSC with heat-treated and non-treated OPSC containing of different species aggregate OPSC at 28-days, 90-days and 180-days are presented in Fig. 6. The results indicate that the heat-treated OPSC had a positive effect on porosity as compared to OPSC without heat-treated at all ages investigated. The reduction of porosity measurement for D0 at 28-days, 90-days and 180-days was about 5.9%, 7.6% and 7.7% as compared to T0. It can be seen that the porosity reduced with an increase in heat treatment temperature from NT to 65 °C/1 h and 130 °C/1 h, respectively, except for T130/1 achieved above 16% at all ages. This may due to the rate of temperature and duration (T130/1) of heat treatment has a negative impact on the properties of tenera OPS. On the other hand, the lowest percentage of porosity measurement shown a positive effect was observed at D65/1. In fact, the reduction in porosity of T65/1 and D130/1 is probably due to the suitable of heat treatment method on OPS species. Therefore, it can be proved that the similar trend of tests on the durability that have performed in this study. Hence, the results approved the validity of RCPT values. It has been reported that the porosity value of normal strength concrete is 17% for compressive strength less than 50 MPa (Khan, 2007). For self-consolidating high performance concrete, the porosity values range between 6 and 14% (Safiuddin et al., 2010). Thus, it is worth noting that the porosity value measured for the concrete in this study falls within the range of the results reported.

3.4.3 Water absorption

![Image of water absorption graph]

Figure 7: Water absorption versus heat-treated and without heat-treated on different species aggregate OPSC
Figure 7 shows the water absorption for all mixes. It can be seen that D0 without heat-treated OPS had the highest water absorption value at about 6.02% while T65/1 with heat-treated dura aggregate OPSC achieved the lowest water absorption of 3.08%. Moreover, it can be observed that the water absorption for tenera heat-treated OPSC is lower compared to dura OPSC without heat-treated. This may be due to the low equilibrium moisture content in OPS when dried wood is subjected to heat treatment (Stamm & Hansen, 1937), and therefore the heat-treatedOPS aggregates tend to reduce the water absorption as compared to OPS without heat-treated. In addition, the inclusion of thinner tenera OPS had a positive effect on water absorption, which can be related to the texture aggregates reduce water absorption. It has been reported that most good concrete have water absorption below 10% by weight (Neville, 2008). Thus, it can be deduced that the water absorption value measured for the OPSC with and without heat-treated OPSC in this study falls within the range of good concrete. Shafigh et al. (2011) reported that the water absorption for high strength (43 - 48 MPa) OPSC is within the range of 3.12 – 6.20%. Yew et al. (2014) also reported that for high strength (40 – 54 MPa) OPSC the water absorption fall within the range of 3.04 – 6.30%. For OPSC with conventional compressive strength (15 – 29 MPa), the water absorption was in the range between 10.64% and 11.23% (Teo et al., 2007).

3.4.5 Drying shrinkage

Figure 8 shows the development of the drying shrinkage of OPSC with heat-treated and non-treated containing different species OPS aggregate. As can be observed that the drying shrinkage curve of heat-treated OPSC is higher than the without heat-treated OPSC at all ages. From the figure, the increasing rate of drying shrinkage of all mixes is typically linear in the first month. However, the magnitude of shrinkage seems to plateau with time. It can be seen that after nearly 240 days of continuous drying, the shrinkage values in T0 and D0 mixes tended to become constant at about 430 micro strain. In contrast, heat-treated OPS species from 65/1 to 130/1 continued to shrink until approximately at 180 days of drying. Shrinkage rate in heat-treated OPSC become very slow after 180 days of continuous drying. The average increment of heat-treated OPSC drying shrinkage value is about 14.5% compared to OPSC without heat-treated at all ages. Furthermore, the increment of heat-treated OPSC significantly increases the drying shrinkage. Based on the 300 days of continuous drying, the D130/1 had achieved the highest micro strain at about 656 as compared to other mixes at all ages. This phenomenon might be attributed to heat treatment on OPS alter the physical properties of aggregate which resulting in better adhesion between cement paste and OPS. It has been reported that a decrease in hygroscopicity of wood tend to reduced shrinkage and enhancement of dimensional stability may occur when dried wood is subjected to heat treatment (Stamm & Hansen, 1937 and Rapp, 2001). In addition, an advantage can be obtained
from including of dura OPS aggregate at 130/1. It could be due to the efficacy of heat-treatment duration on dura species in the arrest of cracking, which results from drying shrinkage (Balaguru & Slattum, 1995 and Berke & Dallaore, 1994).

4. Conclusion

The effects of incorporating of OPS species at proper duration and heat-treatment on the durability properties and drying shrinkage of high-strength oil palm shell lightweight concrete have been investigated in this study. The following conclusions are drawn based on the experimental results:

1) The workability increases by increasing the duration of heat-treated temperature of OPS aggregates. The slump value for T130/1 and D130/1 mix exhibit a very high workability within a range of 202 – 210 mm.

2) The compressive strength of heat-treated and non-treated aggregates on OPSC increased at all ages. However, the compressive strength decreased with an increase in heat treatment temperature and duration at T130/1.

3) The results of RCPT test clarify that, by incorporating heat-treated OPS aggregate into OPSC improves the resistance of concrete to ion penetration which may result of concrete to be more secured from corrosion especially at D65/1.

4) Generally high strength concrete has porosity ranging from 12 to 15%. It is recorded that the porosity of heat-treated and non-treated OPSC is within the range of 11 to 17 %. Addition of OPS aggregate at 65 °C/1 h had a positive effect, which can be related to the pore blocking effect of heat-treated OPS.

5) The water absorption of heat-treated and non-treated OPSC value varies from 3.08 to 6.02% for all mixes, which falls within the range of good concrete.

6) The shrinkage results of OPSC up to 365 days indicated that all heat-treated OPS aggregate exhibited better drying shrinkage than the OPSC without heat-treated.

Author Contributions (as necessary)
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